

Analysis of Spatial and Temporal Distribution of Water Elevation, Speed and Fluxes in Inundation Zone of Long Waves

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Tsunami

- generation mechanisms
- split and evolution
- propagation
- coastal amplification, inundation and runup
- near field impact
- Iong distance propagation and far field impact
- assessment, preparedness and mitigation
- Tsunami models compute certain tsunami parameters according to the inputted initial wave and bathymetric conditions.





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- Tsunami modeling covers;
 - i) mathematical description of the problem and initial/ boundary conditions with proper approximations and assumptions,
 ii) solutions of the governing equations with different techniques,
 - iii) simulation, visualization.
- Beyond these;

analysis and interpretation of the results and tsunami parameters, understanding of their effects in the inundation zone developing the mitigation measures accordingly using them for educational and public awareness purposes.





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Tsunamis can:

1) DRAG AND DAMAGE drag the whole structures or their units at land or vessels in the sea,





4) OVERTURN DAMAGE

overturn structures by suction of receding or thrust of advancing waves.

Tsunamis can:

3) SCOUR-EROSION-DEPOSITION

undercut foundations and pilings with erosion caused by the receding waves,





2) **DEPOSITION**

deposition with accumulated shoreline debris by the advancing wave fronts, (i.e. 5km inland of West Sumatra Meulaboh)

Causes by Tsunami in Shallow and Inundation Zone

- Water level rise and fall
- Strong currents
- Forces (drag, impact, uplift, etc.)
- Sea water withdraw and drawdown
- Scour, and morphological changes (erosion, deposition)
- Debris and debris flow
- Dynamic water pressure
- Resonant oscillations and seiches







$$\frac{\partial \eta}{\partial t} + \frac{\partial M}{\partial x} + \frac{\partial N}{\partial y} = 0$$

$$\frac{\partial M}{\partial t} + \frac{\partial}{\partial x} \left(\frac{M^2}{D}\right) + \frac{\partial}{\partial y} \left(\frac{MN}{D}\right) + gD \frac{\partial \eta}{\partial x} + \frac{gn^2}{D^{7/3}} M \sqrt{M^2 + N^2} = 0$$

$$\frac{\partial N}{\partial t} + \frac{\partial}{\partial x} \left(\frac{MN}{D}\right) + \frac{\partial}{\partial y} \left(\frac{N^2}{D}\right) + gD \frac{\partial \eta}{\partial y} + \frac{gn^2}{D^{7/3}} N \sqrt{M^2 + N^2} = 0$$

Non-linear longwave equations

M, N : Discharge fluxes in x&y directions

n : Manning's roughness coefficient

 $M = u(h+\eta) = uD$ $N = v(h+\eta) = vD$

Specific Terms (nearshore and inundation Zones):

Run-up: The vertical distance of water level from reference sea level at the end of inundation zone

Maximum positive amplitude - Wave height: Vertical distance of water level from reference sea level

Flow depth: Depth of the flow in inundation zone

Current velocity: Velocity of flow in near shore and inundation zone

Discharge flux : Discharge flux in near shore and inundation zone

Inundation distance: Horizontal distance a tsunami reaches landward from shoreline of reference sea level. Maximum negative amplitude: The maximum level of subsidence from

reference sea level.





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1) Harry Yeh (2006) (Drag Force per unit width)

$$F = \frac{1}{2} C_d \rho h u^2$$

C_d: drag coefficient (\approx 1.0 - 2.0 suggested by Arnason (2004)) h : total flow depth u : flow velocity







2) Synolakis & Yalciner (in ASCE LIMAS Sumer 2005)



Hydrostatic Force
$$\rightarrow F_h = \frac{1}{2}\rho_w g d^2 w = \frac{1}{2}\rho_w g d A$$





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2) Synolakis & Yalciner (in ASCE LIMAS Sumer 2005)

Drag Force
$$\rightarrow F_d = \frac{1}{2} C_D \rho_w A u^2$$

C_d: drag coefficient

A: cross-sectional area exposed to drag force

u: flow velocity

- This approach can be applied to compute force of flow onto objects.





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2) Synolakis & Yalciner (in ASCE LIMAS Sumer 2005)

Normalization of Drag Force by Hydrostatic Force



Drag coefficient (shape parameter)



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3) Ramsden & Raichlen (1990)

$$\frac{F_T}{\frac{1}{2}\gamma b(H_1 + d_w)^2} = \left(\frac{\eta + d_w}{H_1 + d_w}\right)^2 + 2C_F N_F^2 \frac{\eta H_1}{(H_1 + d_w)^2}$$

where $C_F = 1 + (\tan \theta)^{1.2}$: force coefficient \approx 1.4-2.1

$$N_F = \frac{c}{\sqrt{gH_1}}$$

→ c: bore celerity (spead of wave shape)

- H₁: surge height
- η : surge profile
- b : width of wall
- d_{w} : water depth at the wall

 After appropriate assumptions and simplifications, the equation becomes:





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Importance of Near shore and Inundation Parameters

- Maximum positive amplitude >>>> governs the intensity of the tsunami
- Maximum current velocity >>>> governs erosion, deposition, debris and drag
- Discharge Flux an instantaneous value depending on water velocity and flow depth. Since current velocity and flow depth are changing independently, the discharge flux also varies with time and location
- Hydrodynamic load \implies an instantaneous value during inundation depending on water velocity and flow depth. Since current velocity and flow depth are changing independently, the hydrodynamic demans also varies with time and location





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MANUAL Tsunami Simulation/Visualization Code NAMI DANCE NESTED DOMAIN version5.9



PDF version is at http://namidance.ce.metu.edu.tr/pdf

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Numerical Modeling

Simulation code

TUNAMI N2 (authored by Profs. Shuto and Imamura) by inserting a new subroutine to compute above mentioned parameters.

Dynamic Wave Input :

- amplitude = 1 m
- period = 3 min
- 2 successive sinusoidal waves
- sea bottom slope = 1/12.5 or 1/50
- wave shape : Leading Depression Wave





wave direction

Numerical Modeling

- Sea bottom topography:
- Horizontal distance =1500 m
- vertical distance = 3000 m
- Delta x = 5 m
- Delta t = 0.1 sec

wave direction







Figure 6: Distribution of maximum positive amplitudes for perpendicular approach of the wave (angle=90°) with period of 3 min. for LEW with the bottom slope of 1/10 (incoming wave amplitudes= +0.5m, +1.0m at the toe)





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Özer and Yalciner (2010)

Figure 7: Distribution of maximum positive amplitudes for perpendicular approach of the wave (angle=90°) with period of 3 min. for LDW with the bottom slope of 1/10

(incoming wave amplitudes= -0.5m, -1.0m at the toe)









Özer and Yalciner (2010)

Figure 10: Distribution of maximum hydrodynamic demands for perpendicular approach of the wave (angle=90°) with period of 3 min. for LDW with the bottom slope of 1/10 (incoming wave amplitudes= -0.5m, -1.0m at the toe)





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Figure 11: Distribution of maximum hydrodynamic demands for perpendicular approach of the wave (angle= 90°) with period of 3 min. for LDW with the bottom slope of 1/20 (incoming wave amplitudes= -0.5m, -1.0m at the toe)























- Rupture Nme: s18-Z22
- Type: Normal
- Epicenter: 26.36E, 37.64N
- Dip : 45°
- Rake : 45°
- Strike : 95°
- Focal Depth(km): 20
- L (km): 95
- W (km): 30
- Displacement u (m): 4



- Maksimum (+) Amplitude at the source (m): 1.80
- Maksimum (-) Amplitude at the source (m): -0.30







• Nested Grid System B, C, D ve E Domains

B, C, D ve E alanlarına ait grid aralıkları ve sınır koordinatları

Alan Adı	Sınır Koordinatları	Grid Aralığı (m)
В	26.1° - 27.8° E 36.9° - 38.0° N	270
С	27° - 27.7° E 37° - 37.5° N	90
D	27.18° - 27.66° E 37.065° - 37.42° N	30
E	27.323° - 27.667° E 37.068° - 37.294° N	10



























27.5

27.6





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Concluding Remarks

 Water elevation (rise) 	sea and land
 Current velocity 	sea and land
 Flow depth 	land
Discharge Flux	land
• Froude number	land
 Water elevation (fall) 	sea

Note: The presentation covered a case of small tsunami. Further investigation and comparisons are necessary by using large size tsunami and different near shore conditions for generalization





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THANKS FOR KIND ATTENTION





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